

Response of Multistorey Structure with and Without of Expansion Gap for Wind Loading and Dynamic Analysis

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Abstract - Now a days Buildings are analysed and designed by the different software such as Staad pro, Etabs, sap 2000 etc. The latest Practice for designing the Buildings are Etabs and Staad pro because of their best user interface. In our case the building we are analysing with the help of Staad pro by the use of Different codes for Earthquake, Wind and Service loads etc. This paper presents an investigation on behaviour of Multi-storey building with and without expansion joint by creating 8 different model which includes the Shear wall, Response spectrum analysis wind analysis etc.

Key Words: Expansion joint, shear wall, Response spectrum analysis, Wind analysis etc.

1. INTRODUCTION

In order to design a structure to resist wind and earthquake loads, the forces on the structure must be specified. The exact forces that will be occur during the life of the structure cannot be anticipated. Most National Building Codes identify some factors according to the boundary conditions of each building considered in the analysis to provide for life safety. Where there is another technique to resist or to minimize the ground motion effect on earthquake is to provide the expansion joints on the building. The codes say that it is compulsory to provide the expansion joint at every 45m if the building is more in length. The term "expansion joint" as used refers to the isolation joints provided within a Building to permit the separate segments of the structural frame to expand and contract in response to Temperature changes without adversely affecting the building's structural integrity or serviceability. The normal practice in runways, bridges, buildings and road construction is to provide expansion joints between cutting slabs of reinforced concrete at designing intervals and at intersections with other constructions. These joint t filers are then covered with sealing compounds. Building expansion joints are used to cover the space between components, and provide a barrier to the exterior. Expansion joints can follow complicated paths along varying materials. Copper is an excellent material for such joints, since it is easy to form and lasts a long time.

2. PRELIMINARY DATA CONSIDERED FOR THE ANALYSIS:

- Area covering = 25m x 84 m.
(As shown in fig 01)
- Total Height of the building = 32 m
- Floor to Floor Height = 3.2m

COLUMN DETAILS:

- Central column = 300x750 mm
- Outer corner column = 300x650 mm
- Outer peripheral column = 300x600 mm

BEAM DETAILS:

- Outer Beams = 300x450 mm
- Internal Beams = 300x530 mm

3. FOLLOWING ARE CODES CONSIDERED FOR THE ANALYSIS:

- R.C.C. design : IS 456: 2000
- Earthquake design : IS1893: 2002
- Code for Dead load : IS875: Part 1
- Code for Live load : IS875: Part 2
- Code for wind load : IS 875: Part 3
1987 & 2015

4. STAAD MODEL PREPARED –

- Model-01 -Without Expansion wind load IS 875:2015
- Model-02 -With Expansion wind load IS 875:2015
- Model-03 -Without Expansion wind load IS 875:1987
- Model-04 -With Expansion wind load IS 875:1987
- Model-05 -Without Expansion wind load shear wall IS 875:2015
- Model-06 -With Expansion wind load shear wall IS 875:2015
- Model-07 -Without Expansion Dynamic analysis IS 875:2015
- Model-08 -With Expansion Dynamic analysis IS 875:2015

5. EARTHQUAKE DETAILS

- ZONE : IV(DELHI)
- ZONE FACTORE : 0.24
- IMPORTANCE FACTOR : 1.5
- TIME PERIOD IN STATIC X : 0.57
- TIME PERIOD IN STATIC Y : 0.31

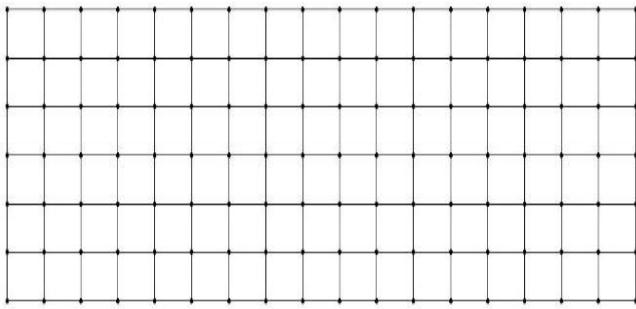


FIG: SHOWS THE PLAN OF THE BUILDING

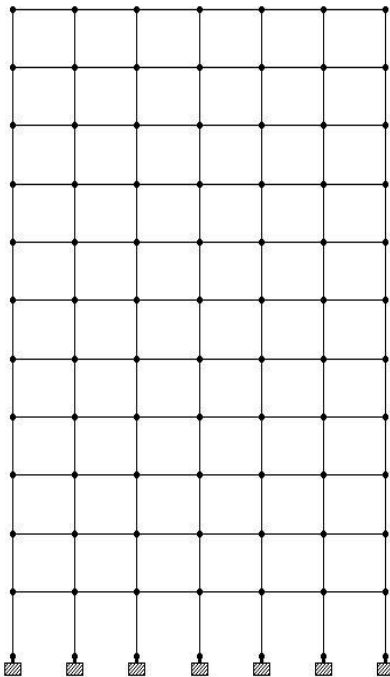


FIG: SHOWS THE ELEVATION

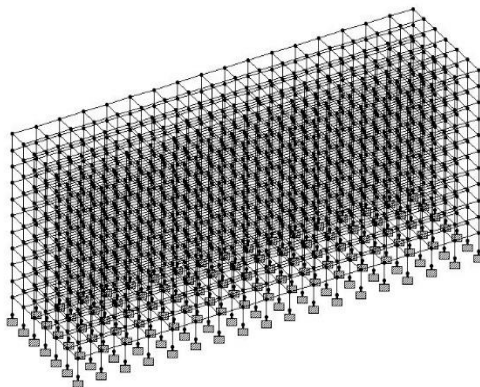


FIG: SHOWS THE 3D MODEL IN STAAD PRO

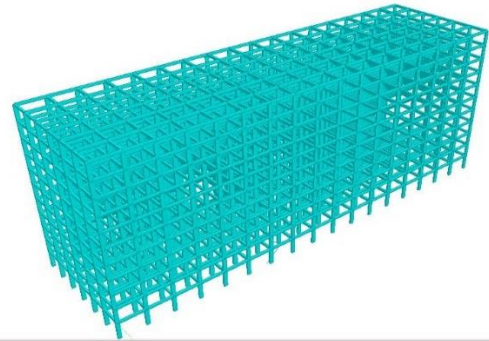


FIG: SHOWS THE EXTRUDED VIEW IN STAAD PRO

6. RESULTS: -

Table1. MODEL TIME PERIOD FOR THE DYNAMIC LOAD CONSIDERATION WITH AND WITHOUT EXPANSION JOINTS

SR.NO	MODE	TIMEPERIOD IN SECONDS WITHOUT EXPANSION JOINT	TIMEPERIOD IN SECONDS WITH EXPANSION JOINTS
1.	MODE 01	1.96	2.34
2.	MODE 02	1.87	2.30
3	MODE 03	1.41	2.11

Table2. BASE SHEAR DETAILS

LOAD CASES	WITHOUT EXPANSION JOINT	WITH EXPANSION JOINT
BASE SHEAR FOR EQX	2111 KN	1838 KN
BASE SHEAR FOR EQZ	1570 KN	1387 KN

Table3. MAXIMUM NODAL DISPLACEMENT WITHOUT EXPANSION JOINT

SR. NO	LOAD CASE	IS 875-3 [1987]	IS 875-3 [2015]	MODEL WITH SHEAR WALL
1.	DL	11.599	11.599	11.59
2.	LL	1.112	1.112	1.108
3	WIND X+VE	3.25	2.27	1.28
4	WIND X-VE	3.25	2.27	1.28
5	WIND Z+VE	21.77	15.208	9.93
6	WIND Z-VE	21.77	15.208	9.93

Table4. MAXIMUM NODAL DISPLACEMENT WITH EXPANSION JOINT.

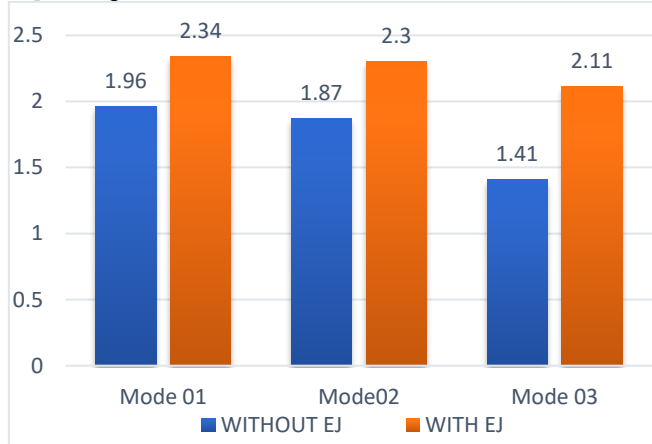
SR. NO	LOAD CASE	IS 875-3 [1987]	IS 875-3 [2015]	MODEL WITH SHEAR WALL
1.	DL	11.599	11.599	5.34
2.	LL	1.113	1.113	0.84
3	WIND X+VE	5.47	3.28	1.43
4	WIND X-VE	7.85	5.48	1.43
5	WIND Z+VE	20.19	14.84	5.78
6	WIND Z-VE	20.19	14.84	5.78

Table5. MAXIMUM NODAL DISPLACEMENT WITH AND WITHOUT EXPANSION JOINT WHILE CONSIDERING DYNAMIC EARTHQUAKE

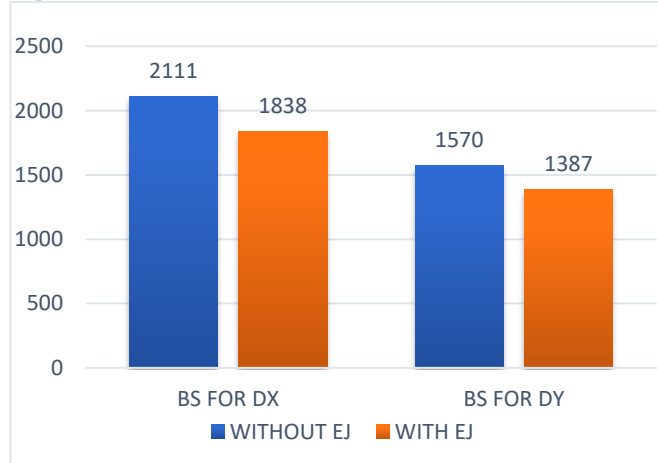
SR. NO	LOAD CASE	MODEL WITH EXPANSION JOINT	MODEL WITHOUT EXPANSION JOINT
1.	DL	11.59	11.59
2.	LL	1.113	1.105
7	EQ X+	11.19	9.19
8	EQ X-	11.19	9.19
9	EQ Z+	15.43	12.78
10	EQ Z-	15.43	12.78

7.GRAPH:

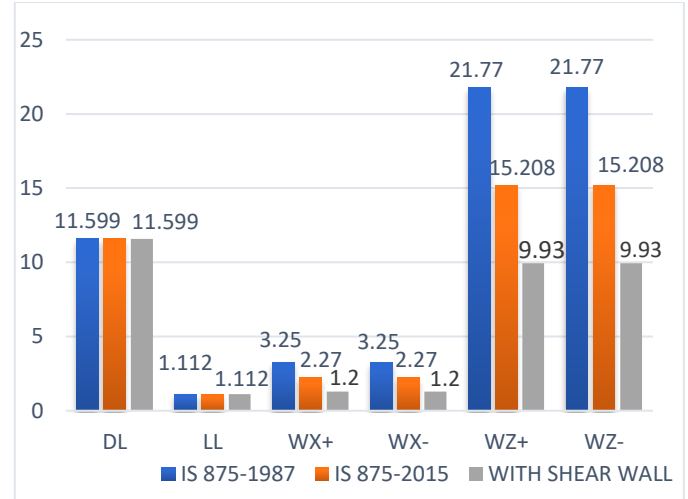
01] Time period



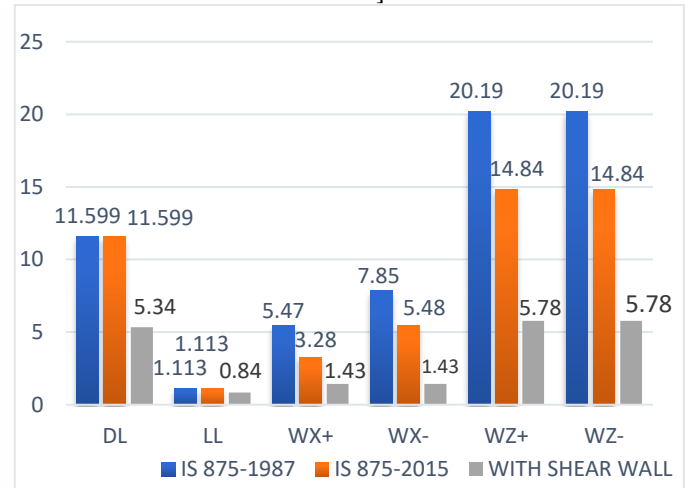
02] Base shear:



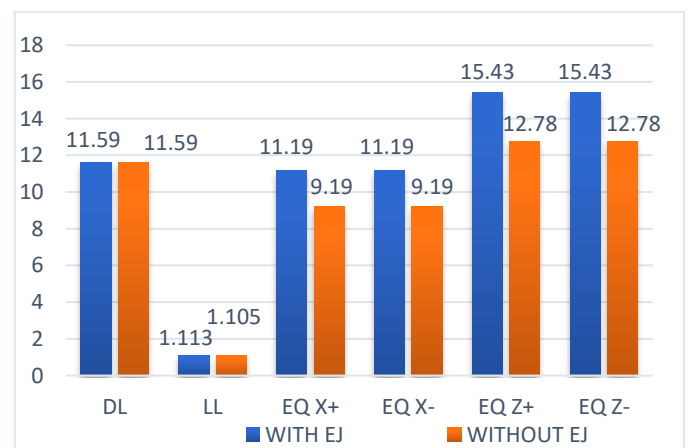
03] NODAL DISPLACEMENT [WITHOUT EXPANSION JOINT]



04] NODAL DISPLACEMENT [WITH EXPANSION JOINT]



05] NODAL DISPLACEMENT [WITH AND WITHOUT EXPANSION JOINT] WHILE CONSIDERING DYNAMIC EARTHQUAKE



8. CONCLUSIONS

Following are the conclusion we have obtained from above analysis results are: -

1. Time period

In case of Time period the values were obtained for without Expansion joint model is 1.96, 1.86, 1.41 in first, second and third mode is lower than the Model time period obtained in model with Expansion joint is [2.34, 2.30, 2.11] for First, second and third mode respectively as shown in table 01. Which means that while using is Expansion joint in the model the building will take more time to oscillate for all three modes when comparing with without expansion joint building model as shown in table 01.

2. Base shear

Base shear values in Static X direction is 2111 kN in Normal building model without expansion joint and for the building model using Expansion joint the base shear values are 1838 kN which means the base shear values are reducing. While using expansion joint in the building model as shown in table 02.

3. Nodal Displacement

A] While considering the IS 875-1987 and 875-2015 for the wind load the obtained results indicate that there is no measure different in the Displacement but the values obtained by using 875-2015 are quite lower than that of IS 875-1987 as shown in table 03, 04 in Wind x and Wind Z direction.

B] In case of Shear wall the Nodal displacement values are getting reduced by almost 50% because the shear wall having the in plane as well as out of plane stiffness due to that the displacement is reduced as shown in the table 03, 04 above.

C] If earthquake is considered in both model of with and without Expansion joint the displacement values are increased by almost 30% in EX, EZ direction in the model where Expansion joints are used as shown in the table 5 above.

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