

# Fluid Induced Piping Vibration with Elastically Restrained Different End Supports\*

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**Abstract**—The dynamic stability of elastically restrained pipe conveying fluid is investigated in this study. The frequency expression is derived for classical boundary conditions by considering the supports as compliant material with linear and rotational stiffness. A new transcendental frequency equation is developed by using Euler-Bernoulli beam theory; the equation of motion is derived from energy expressions using the Hamilton's Principle. The natural frequencies are presented for a wide range of restraint parameters. Cases are studied for different boundary conditions- Linearly and Rotationally Restrained, Rotational-Linearly & Linearly Restrained, Rotational-Linearly and Fixed, Rotationally Restrained –Guided, are computed and it is noted that as the flow velocity increases the first mode frequency decreases and by varying the mass ratio the frequency increases.

**Keywords**—Elastically Restrained, Frequency, Linearly Restrained, Pipe, Guided Support

## I. INTRODUCTION

The vibration analysis of piping systems is important from the view point of safeguarding the equipment's and pipelines from damage which is mostly applicable in chemical, petrochemical and other allied industries. It is well known that pipeline systems may undergo divergence and flutter type of instability due to fluid-structure interaction.

The dynamic behavior of fluid conveying pipes was predicted first by Ashley and Haviland in 1950 [1]. and later by Housner in 1952. Housner considered a simply supported beam model for the pipeline and analysed it using a series solution approach which showed that critical flow velocity could cause buckling [2]. S. S Rao developed a mathematical model for transverse vibration for elastically restrained conditions for beams [3]. Naguleswaran and Williams developed solutions for natural frequencies in axial mode for Hinged-Hinged, Fixed-Hinged and Fixed-Fixed boundary conditions [4]. Chen and Paidoussis developed dynamic stiffness matrix for coupled fluid structure interaction [5], [6]. Huang Yi-min considered Galerkin's method and obtained natural frequencies for fluid conveying pipeline with different boundary conditions [7]. R.A. Stein and M.W.Tobriner discussed Vibration of pipes containing flowing fluid, in which the effects of foundation modulus, flow velocity and internal pressure on the dynamic stability, frequency response and wave

propagation characteristics of an un-damped system was studied [8]. Wang Shizhong, Liu Yulan, Huang Wenhui, had conducted research on solid liquid coupling dynamics of pipe conveying fluid, where they studied the influence of flowing velocity, pressure, solid-liquid coupling damping and solid-liquid coupling stiffness on natural frequency for simply supported ends [9]. Weaver D.S and Unny T.E. studied the dynamic stability of finite length of pipe conveying fluid using Flugge-Kempner equation to find the critical flow velocities [10].

In most of the cases, the differential equation of motion of fluid-conveyed pipe is deduced using the Galerkin's method in Lagrange system. Subsequently, the solution of the differential equation is obtained by considering many numerical methods such as transfer matrix, finite element, perturbation, Runge-Kutta and differential quadrature.

It is the need to have a better understanding of the dynamics of the pipes conveying fluid. The various important factors that influence the dynamic behaviour of fluid conveying pipe are (i) Flow Velocity (ii) Support conditions and (iii) Interaction with supporting medium. Hence, the estimation of exact natural frequencies of pipes is presented with exact approach for finding the transverse vibration of elastically restrained pipes.

## Nomenclature

EI	bending stiffness of a pipe
$m_p$	mass of pipe
U	velocity of the fluid
w	lateral deflection of pipe
$\gamma$	mass ratio
$\lambda$	non-dimensional parameter
$m_f$	fluid mass
t	time
x	the axial coordinate
V	non-dimensional Velocity
$\alpha$ & $\beta$	coefficient of trigonometric function
$\xi$	natural boundary conditions.

II. MATHEMATICAL MODEL

A. FORMULATION OF PROBLEM

The governing differential equation of motion and boundary conditions corresponding to the transverse vibration of pipe has been derived by considering the equilibrium approach. Consider a straight uniform single span pipe conveying fluid of length L where K<sub>1</sub> and K<sub>2</sub> are translational and kt<sub>1</sub> and kt<sub>2</sub> are rotational stiffness parameters as shown in Figure1. The displacement of the pipe is assumed to be restrained in the z-x plane.

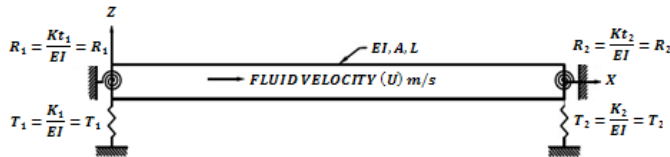


Fig.1. Elastically restrained pipe conveying fluid and with both ends supported

Where m<sub>p</sub> is the mass per unit length of pipe, m<sub>f</sub> is the mass of fluid per unit length, U is the flow velocity, E is the elastic modulus of the pipeline material, I is the moment of inertia of cross-section of the pipe and z is the lateral deflection of the pipeline; x and t are the axial co-ordinate and time, respectively. Considering only time dependent variables, and after simplification, we obtain the following transverse vibration equation of the pipeline conveying fluid.

From first principles and applying the Euler-Bernoulli beam theory and Hamilton's energy equations for the elastically restrained pipe conveying fluid, the differential equation of motion and boundary conditions are obtained as (3)

$$EI \frac{\partial^4 w}{\partial x^4} + (m_p + m_f) \frac{\partial^2 w}{\partial t^2} + m_f U^2 \frac{\partial^2 w}{\partial x^2} + 2m_f U \frac{\partial^2 w}{\partial x \partial t} = 0 \quad (1)$$

EI = Bending stiffness of a pipe;

m<sub>p</sub> + m<sub>f</sub> = mass of pipe and mass of fluid;

U = Velocity of the fluid; t = time;

$EI \frac{\partial^4 w}{\partial x^4}$  is stiffness term;

$(m_p + m_f) \frac{\partial^2 w}{\partial t^2}$  inertia force term;

$m_f U^2 \frac{\partial^2 w}{\partial x^2}$  curvature term;  $2m_f U \frac{\partial^2 w}{\partial x \partial t}$  coriolis force term.

The boundary conditions for the piping system are given below

$$\left[ \left( -EI \frac{\partial^2 w}{\partial x^2} \right) \right]_{x=0} = 0 \quad (2)$$

$$\left[ \left( EI \frac{\partial^3 w}{\partial x^3} - T^* \frac{\partial w}{\partial x} \right) \right]_{x=0} = 0 \quad (3)$$

$$\left[ \left( EI \frac{\partial^2 w}{\partial x^2} \right) \right]_{x=L} = 0 \quad (4)$$

$$\left[ \left( -EI \frac{\partial^3 w}{\partial x^3} + T^* \frac{\partial w}{\partial x} \right) \right]_{x=L} = 0 \quad (5)$$

Where  $T^* = \alpha^2 - \beta^2$  and L=Length of the pipe supports or span.

The equation of motion Eq. (1) can be written in the following non-dimensional form:

$$\frac{\partial^4 w}{\partial \xi^4} + (V^2) \frac{\partial^2 w}{\partial \xi^2} + 2\gamma U \frac{\partial^2 w}{\partial \xi \partial t} - \lambda^4 w = 0 \quad (6)$$

Where,

$\gamma$  = Mass ratio,  $\xi$  = Natural boundary condition,

$\omega$  = Natural frequency of pipe vibration,

V = Non-dimensional velocity and  $\lambda$  = Wavelength

$$V^2 = \left( \frac{m_f U^2 L^2}{EI} \right); \quad \lambda^4 = \left( \frac{(m_p + m_f) \omega^2 L^4}{EI} \right)$$

$$\frac{\partial^4 w}{\partial \xi^4} + (V^2) \frac{\partial^2 w}{\partial \xi^2} - \lambda^4 w = 0 \quad (7)$$

$$2c^2 = (V^2); \quad 2c = \sqrt{V^2}$$

When the natural frequency of the pipe approaches zero the critical flow velocity has been computed for all the end conditions. When the flow velocity is equal to the critical velocity, the pipe bows out and buckles, as the forces required to make the fluid deform to the pipe curvature are greater than the stiffness of the pipe. The term Coriolis force represents the damping of the system, and its effect on the frequency of vibration is negligible and so is omitted, as the present work aims to obtain upper bounds for the frequencies of vibration of the pipe conveying fluid. The damping term is omitted and Eq. (7) is a non-dimensional partial differential equation of higher order with boundary problem.

Now let  $W(x) = c \cdot e^{sx}$

(8)

$$\frac{\partial^4 w(x)}{\partial \xi^4} = c \cdot s^4 \cdot e^{sx}; \quad \frac{\partial^2 w}{\partial \xi^2} = c \cdot s^2 \cdot e^{sx} \quad (9)$$

$$c \cdot e^{s^4} + 2c \cdot c^2 \cdot s^2 e^{sx} + \lambda^4 \cdot c \cdot e^{sx} = 0$$

$s^4 + 2c^2 \cdot s^2 + \lambda^4 = 0$ , where c and s are constants and

substitution of eq. (8) in to (7) results.

$$(s^2)^2 + 2c^2 \cdot s^2 + (\lambda^2)^2 = 0 \quad (10)$$

The roots of equation (10) is given by

$$s^2 = \frac{-2c^2 \pm \sqrt{(2c^2)^2 - 4\lambda^4}}{2}$$

$$s^2 = -c^2 \pm \sqrt{c^4 - \lambda^4}$$

$$s_1 = \alpha; \quad s_2 = i\beta; \quad s_3 = -\alpha; \quad s_4 = -i\beta$$

Considering first two roots  $s_1 = \alpha$  and  $s_2 = i\beta$

$$\text{Then } \alpha = \sqrt{-c^2 + \sqrt{c^4 + \lambda^4}}; \quad \beta = \sqrt{c^2 + \sqrt{c^4 + \lambda^4}} \quad (11)$$

**B. NATURAL FREQUENCY EVALUATION**

Let the solution of the general equation (6) given as

$$W(x) = A \sinh ax + B \cosh ax + C \sin \beta x + D \cos \beta x \quad (12)$$

$$\frac{\partial W(x)}{\partial x} = A\alpha \cosh ax + B\alpha \sinh ax + C\beta \cos \beta x - D\beta \sin \beta x \quad (13)$$

$$\frac{\partial^2 W(x)}{\partial x^2} = A\alpha^2 \sinh ax + B\alpha^2 \cosh ax - C\beta^2 \sin \beta x - D\beta^2 \cos \beta x \quad (14)$$

$$\frac{\partial^3 W(x)}{\partial x^3} = A\alpha^3 \cosh ax + B\alpha^3 \sinh ax - C\beta^3 \cos \beta x + D\beta^3 \sin \beta x \quad (15)$$

The above boundary conditions (2), (3), (4) and (5) are substituted in exact solution equations (12) to (15) to get the transcendental frequency equation can be written as follows

$$\begin{aligned} &2\alpha\beta(\alpha^2\beta^2 + R_1T_1)(\alpha^2\beta^2 + R_2T_2) \\ &+ \alpha\beta[(\alpha^4 + \beta^4)(R_1 + R_2)(T_1 + T_2) \\ &+ 2\alpha^2\beta^2(R_1T_2 + R_2T_1) - 2(\alpha^4\beta^4 \\ &+ R_1R_2T_1T_2)] \cosh(\alpha L) \cos(\beta L) \\ &- \alpha(\alpha^2 + \beta^2)[(\alpha^4\beta^2 - T_1T_2)(R_1 + R_2) \\ &+ \beta^2(\beta^2 - R_1R_2)(T_1 \\ &+ T_2)] \cosh(\alpha L) \sin(\beta L) \\ &- \beta(\alpha^2 + \beta^2)[(\alpha^2\beta^4 + T_1T_2)(R_1 + R_2) \\ &- \alpha^2(\alpha^2 + R_1R_2)(T_1 \\ &+ T_2)] \sinh(\alpha L) \cos(\beta L) \\ &- [\alpha^2\beta^2(\alpha^6 - \beta^6) \\ &+ (\alpha^2 + \beta^2)^2(\alpha^2\beta^2R_1R_2 - T_1T_2) \\ &- \alpha^2\beta^2(\alpha^2 - \beta^2)(R_1T_1 + R_2T_2) \\ &- (\alpha^2 - \beta^2)R_1R_2T_1T_2] \sinh(\alpha L) \sin(\beta L) \\ &= 0 \end{aligned} \quad (16)$$

1.8	2.7464	3.0968	3.3162	7.7	2.2363	2.6561	2.9096
1.9	2.7434	3.0941	3.3137	7.8	2.2205	2.6428	2.8975
2.0	2.7402	3.0913	3.3110	7.9	2.2044	2.6293	2.8829
2.1	2.7368	3.0883	3.3082	8.0	2.1879	2.6156	2.8728
2.2	2.7332	3.0851	3.3053	8.1	2.1712	2.6016	2.8600
2.3	2.7295	3.0818	3.3022	8.2	2.1540	2.5874	2.8471
2.4	2.7256	3.0784	3.2990	8.3	2.1366	2.5729	2.8340
2.5	2.7215	3.0748	3.2957	8.4	2.1187	2.5581	2.8206
2.6	2.7173	3.0711	3.2922	8.5	2.1005	2.5431	2.8070
2.7	2.7128	3.0672	3.2885	8.6	2.0819	2.5278	2.7932
2.8	2.7082	3.0631	3.2848	8.7	2.0630	2.5122	2.7791
2.9	2.7035	3.0589	3.2809	8.8	2.0436	2.4964	2.7649
3.0	2.6985	3.0545	3.2768	8.9	2.0238	2.4803	2.7503
3.1	2.6934	3.0500	3.2726	9.0	2.0036	2.4638	2.7355
3.2	2.6881	3.0453	3.2682	9.1	1.9830	2.4471	2.7205
3.3	2.6826	3.0405	3.2638	9.2	1.9619	2.4301	2.7052
3.4	2.6770	3.0355	3.2591	9.3	1.9403	2.4128	2.6897
3.5	2.6711	3.0304	3.2543	9.4	1.9182	2.3951	2.6739
3.6	2.6651	3.0251	3.2494	9.5	1.8957	2.3771	2.6579
3.7	2.6589	3.0196	3.2443	9.6	1.8726	2.3588	2.6415
3.8	2.6525	3.0140	3.2391	9.7	1.8490	2.3404	2.6249
3.9	2.6459	3.0082	3.2338	9.8	1.8249	2.3212	2.6080
4.0	2.6391	3.0023	3.2282	9.9	1.8002	2.3019	2.5909
4.1	2.6322	2.9962	3.2226	10.0	1.7748	2.2822	2.5734
4.2	2.6250	2.9899	3.2168	10.1	1.7488	2.2621	2.5556
4.3	2.6177	2.9835	3.2108	10.2	1.7222	2.2416	2.5376
4.4	2.6101	2.9769	3.2047	10.3	1.6949	2.2207	2.5192
4.5	2.6024	2.9701	3.1984	10.4	1.6669	2.1995	2.5005
4.6	2.5944	2.9632	3.1920	10.5	1.6381	2.1778	2.4815
4.7	2.5863	2.9561	3.1854	10.6	1.6085	2.1556	2.4621
4.8	2.5780	2.9488	3.1786	10.7	1.5780	2.1331	2.4424
4.9	2.5694	2.9414	3.1718	10.8	1.5467	2.1100	2.4223
5.0	2.5607	2.9338	3.1647	.	.	.	.
5.1	2.5517	2.9260	3.1575	.	.	.	.
5.2	2.5425	2.9180	3.1501	12.9	-	0.11260	0.9678
5.3	2.5332	2.9098	3.1426	.	.	0.10649	0.8940
5.4	2.5236	2.9015	3.1349	.	.	.	.
5.5	2.5138	2.8930	3.1270	14.6	.	-	0.7223
5.6	2.5037	2.8843	3.1190	.	.	.	0.6177
5.7	2.4935	2.8754	3.1108	.	.	.	.
5.8	2.4830	2.8664	3.1024	.	.	.	.
5.9	2.4723	2.8571	3.0939	15.6	.	.	-

Equation (16) is the general frequency equation of elastically restrained pipe conveying fluid.

Assuming C = 0 then  $\alpha = \beta = \lambda$

a) Applying the B.C,  $T_1 = T_2 = T$ ,  $R_1 = R_2 = R$ , in the general frequency equation (11), for

Linearly & Rotationally Restrained End Condition (Reference Fig.1) will result as

$$\begin{aligned} &2\alpha\beta(\alpha^2\beta^2 + RT)^2 + \alpha\beta[(\alpha^4 + \beta^4)(4RT) + 4\alpha^2\beta^2RT - \\ &2(\alpha^4\beta^4 + R^2T^2)] \cosh ax. \cos \beta x - \alpha(\alpha^2 + \beta^2)[(\alpha^4\beta^2 - \\ &T^2)(2R) + \beta^2(\beta^2 - R^2)2T] \cosh ax. \sin \beta x - \beta(\alpha^2 + \\ &\beta^2)[(\alpha^2\beta^2 + T^2)2R - \alpha^2(\beta^2 + R^2)2T] \sinh ax. \cos \beta x - \\ &[\alpha^2\beta^2(\alpha^6 - \beta^6)(\alpha^2\beta^2R^2 - T^2) - \alpha^2\beta^2(\alpha^2 - \beta^2)2RT - \\ &(\alpha^2 - \beta^2)R^2T^2] \sinh ax. \sin \beta x = 0 \end{aligned} \quad (17)$$

V	$\Omega_1 @ \gamma_1$	$\Omega_1 @ \gamma_2$	$\Omega_1 @ \gamma_3$	V	$\Omega_1 @ \gamma_1$	$\Omega_1 @ \gamma_2$	$\Omega_1 @ \gamma_3$
0.1	2.7729	3.1203	3.3381	6.0	2.4613	2.8477	3.0852
0.2	2.7727	3.1201	3.3379	6.1	2.4502	2.8381	3.0763
0.3	2.7723	3.1197	3.3375	6.2	2.4388	2.8282	3.0673
0.4	2.7717	3.1192	3.3371	6.3	2.4271	2.8182	3.0581
0.5	2.7710	3.1186	3.3365	6.4	2.4152	2.8080	3.0487
0.6	2.7701	3.1178	3.3357	6.5	2.4030	2.7976	3.0391
0.7	2.7690	3.1168	3.3348	6.6	2.3906	2.7870	3.0294
0.8	2.7678	3.1157	3.3338	6.7	2.3780	2.7761	3.0194
0.9	2.7664	3.1145	3.3327	6.8	2.3651	2.7651	3.0093
1.0	2.7648	3.1131	3.3314	6.9	2.3519	2.7539	2.9990
1.1	2.7631	3.1116	3.3300	7.0	2.3384	2.7424	2.9885
1.2	2.7612	3.1099	3.3284	7.1	2.3247	2.7307	2.9778
1.3	2.7592	3.1081	3.3267	7.2	2.3107	2.7189	2.9669
1.4	2.7570	3.1062	3.3249	7.3	2.2964	2.7068	2.9559
1.5	2.7546	3.1040	3.3229	7.4	2.2818	2.6944	2.9446
1.6	2.7520	3.1018	3.3208	7.5	2.2670	2.6819	2.9331
1.7	2.7493	3.0994	3.3186	7.6	2.2518	2.6691	2.9215

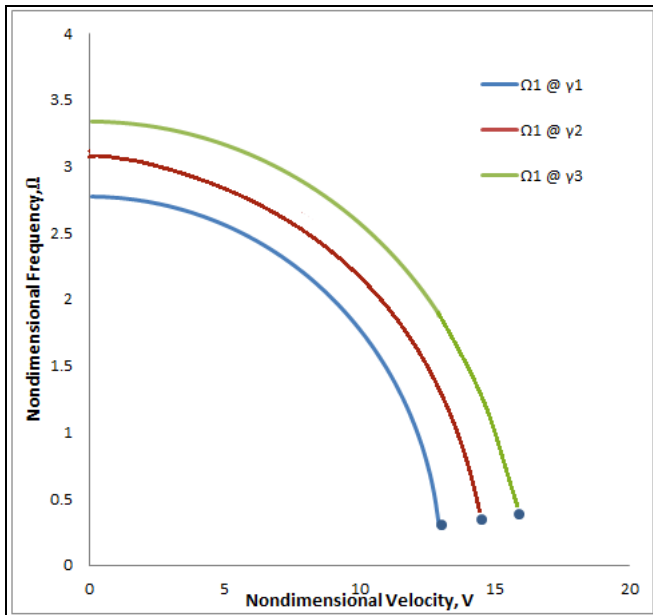


Fig.2. Linearly and Rotationally Restrained End Conditions

b) Applying the B.C,  $T_1 = T_1, T_2 = T_2, R_1 = R_1, R_2 = 0$ , in the general frequency equation [16],

for Rotationally Restrained-Linearly Restrained and Linearly Restrained Condition will result as

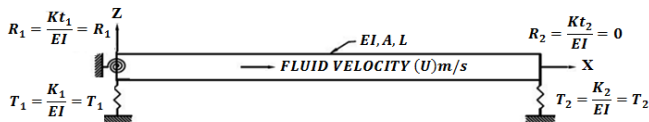


Fig.3. Rotationally Restrained –Linearly Restrained and Linearly Restrained End Condition

$$2\alpha^3\beta^2R_1 + \alpha\beta[(\alpha^4 + \beta^4)R_1] \text{Cosh}\alpha x. \text{Cos}\beta x - \alpha(\alpha^2 + \beta^2)[\beta^4 - R_1T_2] \text{Cosh}\alpha x. \text{Sin}\beta x + \beta(\alpha^2 + \beta^2)[\alpha^4 - R_1T_2] \text{Sin}\alpha x. \text{Cos}\beta x + [(\alpha^2 + \beta^2)^2T_2 + \alpha^2\beta^2(\alpha^2 - \beta^2)R_1] \text{Sin}\alpha x. \text{Sin}\beta x = 0 \tag{18}$$

2.0	2.8303	2.8794	2.9118	3.4	2.4585	2.5320	2.5792
2.1	2.8083	2.8585	2.8916	3.5	2.4246	2.5011	2.5499
2.2	2.7857	2.8371	2.8709	3.6	2.3893	2.4689	2.5196
2.3	2.7626	2.8153	2.8499	3.7	2.3522	2.4355	2.4882
2.4	2.7389	2.7929	2.8283	3.8	2.3134	2.4006	2.45550
2.5	2.7146	2.7699	2.8062	3.9	2.2724	2.3641	2.4215
2.6	2.6895	2.7464	2.7836	4.0	2.2292	2.3258	2.3859
2.7	2.6638	2.7223	2.7605	4.1	2.1832	2.2856	2.3488
2.8	2.6373	2.6975	2.7367	4.2	2.1342	2.2431	2.3097
2.9	2.6373	2.6720	2.7123	4.3	2.0815	2.1980	2.2686
3.0	2.5818	2.6457	2.6872	4.4	2.0245	2.1500	2.2251
0.1	3.1754	3.2105	3.2340	4.5	1.9622	2.0985	2.1789
0.2	3.1599	3.1955	3.2193	4.6	1.8933	2.0429	2.1295
0.3	3.1441	3.1802	3.2044	4.7	1.8160	1.9824	2.0765
0.4	3.1281	3.1648	3.1893	4.8	1.7273	1.9158	2.0190
0.5	3.1119	3.1491	3.1740	4.9	1.6224	1.8414	1.9562
0.6	3.0954	3.1332	3.1585	5.0	1.4919	1.7567	1.8866
0.7	3.0786	3.1170	3.1427	5.1	1.3140	1.6576	1.8084
0.8	3.0615	3.1006	3.1267	5.2	1.0018	1.5367	1.7185
0.9	3.0442	3.0839	3.1104	5.3	-	1.3777	1.6117
1.0	3.0266	3.0669	3.0939	5.4	-	1.1297	1.4782
1.1	3.0086	3.0497	3.0771	5.5	-	-	1.2937
1.2	2.9903	3.0321	3.0600	5.6	-	-	0.9536
1.3	2.9717	3.0143	3.0426	5.7	-	-	-
1.4	2.9527	2.9961	3.0249	-	-	-	-

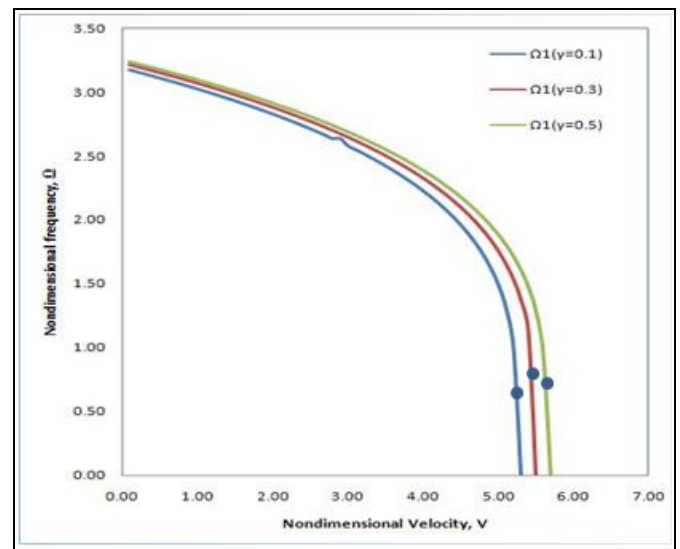


Fig.4. Rotationally Restrained-Linearly Restrained and Linearly Restrained

c) Applying the B.C,  $R_1 = R_1, R_2 = \infty, T_1 = T_1, T_2 = 0$ , in the general frequency equation [16],

for Rotationally Restrained-Linearly Restrained and Fixed End Condition will result as

$$\alpha\beta[(\alpha^4 + \beta^4)T_1 + 2\alpha^2\beta^2T_1] \text{Cosh}\alpha x. \text{Cos}\beta x - \alpha(\alpha^2 + \beta^2)[\alpha^4\beta^2 - \beta^2R_1T_1] \text{Cosh}\alpha x. \text{Sin}\beta x - \beta(\alpha^2 + \beta^2)[\alpha^2\beta^4 - \alpha^2R_1T_1] \text{Sin}\alpha x. \text{Cos}\beta x - [(\alpha^2 + \beta^2)^2\alpha^2\beta^2R_1] \text{Sin}\alpha x. \text{Sin}\beta x = 0 \tag{19}$$

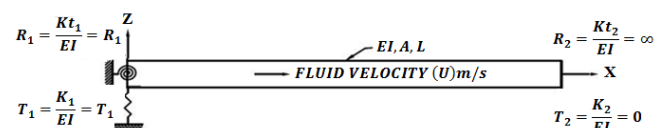


Fig.5. Rotationally Restrained –Linearly Restrained and Fixed End Condition

Table II. Natural Frequencies (N=1)  $\Omega_1$  for Mass Ratios  $\gamma_1 = \sqrt{0.1}, \gamma_2 = \sqrt{0.3}$  and  $\gamma_3 = \sqrt{0.5}$  for Rotationally Restrained –Linearly Restrained and linearly Restrained End Condition

V	$\Omega_1$ @ $\gamma_1$	$\Omega_1$ @ $\gamma_2$	$\Omega_1$ @ $\gamma_3$	V	$\Omega_1$ @ $\gamma_1$	$\Omega_1$ @ $\gamma_2$	$\Omega_1$ @ $\gamma_3$
0.1	3.1754	3.2105	3.2340	1.5	2.9333	2.9776	3.0069
0.2	3.1599	3.1955	3.2193	1.6	2.9136	2.9587	2.9886
0.3	3.1441	3.1802	3.2044	1.7	2.8934	2.9395	2.9700
0.4	3.1281	3.1648	3.1893	1.8	2.8728	2.9199	2.9510
0.5	3.1119	3.1491	3.1740	1.9	2.8518	2.8998	2.9316
0.6	3.0954	3.1332	3.1585	2.0	2.8303	2.8794	2.9118
0.7	3.0786	3.1170	3.1427	2.1	2.8083	2.8585	2.8916
0.8	3.0615	3.1006	3.1267	2.2	2.7857	2.8371	2.8709
0.9	3.0442	3.0839	3.1104	2.3	2.7626	2.8153	2.8499
1.0	3.0266	3.0669	3.0939	2.4	2.7389	2.7929	2.8283
1.1	3.0086	3.0497	3.0771	2.5	2.7146	2.7699	2.8062
1.2	2.9903	3.0321	3.0600	2.6	2.6895	2.7464	2.7836
1.3	2.9717	3.0143	3.0426	2.7	2.6638	2.7223	2.7605
1.4	2.9527	2.9961	3.0249	2.8	2.6373	2.6975	2.7367
1.5	2.9333	2.9776	3.0069	2.9	2.6373	2.6720	2.7123
1.6	2.9136	2.9587	2.9886	3.0	2.5818	2.6457	2.6872
1.7	2.8934	2.9395	2.9700	3.1	2.5526	2.6187	2.6614
1.8	2.8728	2.9199	2.9510	3.2	2.5224	2.5908	2.6349
1.9	2.8518	2.8998	2.9316	3.3	2.4911	2.5619	2.6075

Table III. Natural Frequencies ( $N=1$ )  $\Omega_{1,2,3}$  for Mass Ratio  $\gamma_1 = \sqrt{0.1}$ ,  $\gamma_2 = \sqrt{0.3}$  and  $\gamma_3 = \sqrt{0.5}$  for Rotationally Restrained and Linearly Restrained and Fixed End Condition

V	$\Omega_1 @ \gamma_1$	$\Omega_1 @ \gamma_2$	$\Omega_1 @ \gamma_3$	V	$\Omega_1 @ \gamma_1$	$\Omega_1 @ \gamma_2$	$\Omega_1 @ \gamma_3$
0.1	3.1754	3.2105	3.2340	2.7	2.6638	2.7223	2.7605
0.2	3.1599	3.1955	3.2193	2.8	2.6373	2.6975	2.7367
0.3	3.1441	3.1802	3.2044	2.9	2.610	2.6720	2.7123
0.4	3.1281	3.1648	3.1893	3.0	2.5818	2.6457	2.6872
0.5	3.1119	3.1491	3.1740	3.1	2.5526	2.6187	2.6614
0.6	3.0954	3.1332	3.1650	3.2	2.5224	2.5908	2.6349
0.7	3.0786	3.1170	3.1427	3.3	2.4911	2.5619	2.6075
0.8	3.0615	3.1006	3.1267	3.4	2.4585	2.5320	2.5792
0.9	3.0442	3.0839	3.1104	3.5	2.4246	2.5011	2.5499
1.0	3.0266	3.0669	3.0939	3.6	2.3893	2.4689	2.5196
1.1	3.0086	3.0497	3.0771	3.7	2.3522	2.4355	2.4882
1.2	2.9903	3.0321	3.0600	3.8	2.3134	2.4006	2.4555
1.3	2.9717	3.0143	3.0426	3.9	2.2724	2.3641	2.4215
1.4	2.9527	2.9961	3.0249	4.0	2.2292	2.3258	2.3859
1.5	2.9333	2.9776	3.0069	4.1	2.1832	2.2856	2.3488
1.6	2.9136	2.9587	2.9886	4.2	2.1342	2.2856	2.3097
1.7	2.8934	2.9395	2.9700	4.3	2.0815	2.1980	2.2686
1.8	2.8728	2.9199	2.9510	4.4	2.0245	2.1500	2.2251
1.9	2.8518	2.8998	2.9316	4.5	1.9622	2.0985	2.1789
2.0	2.8303	2.8794	2.9118	.	.	.	.
2.1	2.8083	2.8585	2.8916	5.1	-	1.6042	1.7679
2.2	2.7857	2.8371	2.8709	5.2		1.5367	1.7185
2.3	2.7626	2.8153	2.8499	.	.	.	.
2.4	2.7389	2.7929	2.8283	5.3		-	1.5093
2.5	2.7146	2.7699	2.8062	5.4		.	1.4782
2.6	2.6895	2.7464	2.7836	5.5			-

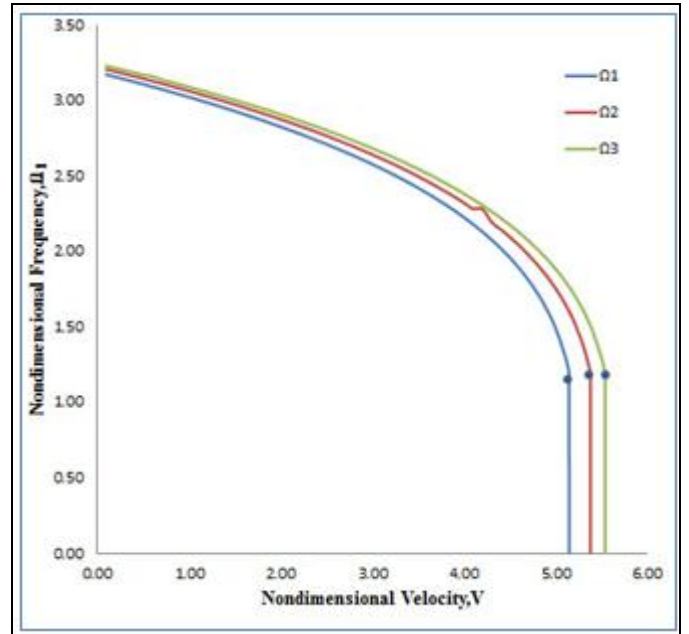


Fig. 6. Rotationally Restrained and Linearly Restrained and Fixed End Condition

d) Applying the B.C,  $R_1 = R_1$ ,  $R_2 = \infty$ ,  $T_1 = 0$ ,  $T_2 = \infty$ , in the general frequency equation [16],

for Rotationally Restrained and Guided End Condition will result as

$$2\alpha^2\beta^2 + (\alpha^4 + \beta^4)\text{Cosh}\alpha x \cdot \text{Cos}\beta x + \beta(\alpha^2 + \beta^2)R_1\text{Cosh}\alpha x \cdot \text{Sin}\beta x + \alpha(\alpha^2 + \beta^2)R_1\text{Sinh}\alpha x \cdot \text{Cos}\beta x - \alpha\beta(\alpha^2 - \beta^2)\text{Sinh}\alpha x \cdot \text{Sin}\beta x = 0 \quad (20)$$

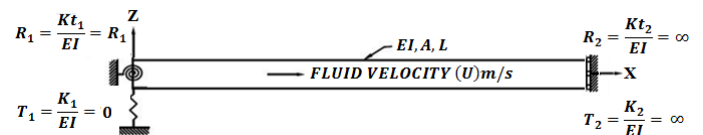


Fig. 7. Rotationally Restrained and Guided End Condition

Table IV. Natural Frequencies ( $N=1$ )  $\Omega_{1,2,3}$  for Mass Ratio  $\gamma_1 = \sqrt{0.1}$ ,  $\gamma_2 = \sqrt{0.3}$  and  $\gamma_3 = \sqrt{0.5}$  for Rotationally Restrained and Guided End Condition

V	$\Omega_1 @ \gamma_1$	$\Omega_1 @ \gamma_2$	$\Omega_1 @ \gamma_3$
0.1	1.8577	1.8336	1.8156
0.2	1.8665	1.8445	1.8269
0.3	1.8740	1.8547	1.8381
0.4	1.8800	1.8639	1.8488
0.5	1.8843	1.8718	1.8586
0.6	1.8869	1.8782	1.8672
0.7	1.8875	1.8831	1.8746
0.8	1.8862	1.8863	1.8804
0.9	1.8827	1.8875	1.8846
1.0	1.8768	1.8868	1.8870
1.1	1.8685	1.8840	1.8875
1.2	1.8573	1.8789	1.8860
1.3	1.8429	1.8714	1.8823
1.4	1.8250	1.8611	1.8762
1.5	1.8026	1.8478	1.8676
1.6	-	1.8311	1.8561
1.7		1.8102	1.8415
1.8		-	1.8231
1.9			1.8003
2.0			-

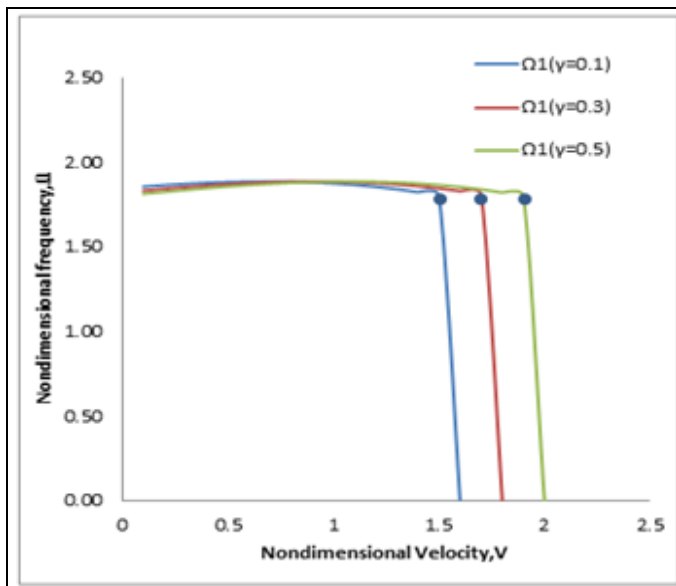


Fig.8. Rotationally Restrained and Guided End Condition

### III. RESULTS AND DISCUSSIONS

Table 1, shows the graph of non-dimensional natural frequencies with non-dimensional velocities. In the case of a) Linearly and Rotationally Restrained end conditions the instability region lies in the range of 12.99435 to 15.66188. However, in other three cases like b) Rotationally Restrained –Linearly Restrained and Linearly Restrained end conditions c) Rotationally Restrained and Linearly Restrained and Fixed end conditions d) Rotationally Restrained and Guided end conditions are shown in Tables 2, 3 & 4 the pipe flutters at a much lower velocity, in the flow region of b)  $V=5.3$  to  $V=5.7$ , c)  $V=5.147$  to  $V=5.338$  and d)  $V=1.6$  to  $V=2.0$ . Figures 2, 4, 6 and 8 shows the points of flutter for three mass ratios. The percentage reduction in frequency as velocity increases from (reference fig.2)  $V=0.1$  to  $V=2.77298$  is 72 %, For Tables 2 and 3 (reference fig.4 & 6) from  $V=0.1$  to  $V=3.175468$  shows 68.24%, reduction in frequency. Table 4 shows the frequency reduction (reference fig.8) from  $V=0.1$  to  $V=1.85777$  is 81.42%. It is found that the natural frequencies remain same for all the three mass ratios, which means that the instability condition is close with higher mass ratio and fluid velocity.

### IV. CONCLUSIONS

- Exact method is developed for pipes conveying fluid for Linearly Restrained and Rotationally restrained end conditions, Rotationally Restrained –Linearly Restrained and Linearly Restrained end conditions, Rotationally Restrained –Linearly Restrained and Fixed end conditions and Rotationally Restrained and Guided end conditions
- The frequencies of the first mode of vibration are computed by varying the fluid velocity
- Critical velocity for different mass ratios are found
- A FORTRAN program is developed for computation of natural frequencies by using Mueller's Iteration method for non-linear equations (Bi-section) and the iterated value of  $x$  (non-dimensional) natural frequency is found by the Inverse Parabolic Interpolation method
- The natural frequencies are obtained by varying the fluid velocities.

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