

Effective Utilization of Reclaimed Asphalt Pavement by using Rejuvenator in Stone Mastic Asphalt For Binder Course

Anusha T M ¹, Sadashiv Avaradi ², and H S Jagadeesh ³

¹Department of Civil Engineering, BMS College of Engineering, Bangalore, India

²Department of Civil Engineering, BMS College of Engineering, Bangalore, India

³Department of Civil Engineering, BMS College of Engineering, Bangalore, India

Abstract:- Road construction crossing the world created huge demand for construction materials. Natural aggregates is the prime material for the road construction use of aggregates leads to question about the preservation of natural resource. In other hand use of recycled pavement asphalt (RAP) as an alternative material in place natural aggregate which makes environmental friendly construction material. This research investigated the feasibility of using reclaimed asphalt pavement in stone mastic asphalt mix by modifying the binder with waste engine oil and elastomer modifier. RAP is replaced with aggregates at 10%, 20%, 30% & 40%. Further in order to increase the percentage of RAP in SMA mix waste engine oil is added to binder course i.e., RAP of 50%, 60% and 70%. The results indicated that 60% of RAP is found to be feasible through laboratory investigation for SMA mixes and 70% RAP is found to be feasible to use in SMA mix through laboratory investigation by modifying binder with waste engine oil.

Keywords: RAP, Bituminous mix, Waste Engine oil, Fatigue, Rutting.

INTRODUCTION

The India road network evolved tremendously in 5 decades to become the main transportation system in the country. India is the second largest country by area in terms of geographical area (32 million square km) and it takes fourth place in world in terms of motor vehicle (3.8 million). But it has very small paved roads with respect to overall area, which in turns leads to increase in transportation cost in the world market.

With more number of vehicles or traffic volume and load of vehicle on the pavement. It becomes very important to provide a pavement meeting the required standards. In order construct the road highly durable, safe, with good riding comfort. By using high-durable asphalt mixtures which will help in reducing the vehicle operation cost and maintenance cost (pavement service life). In developing countries above mention parameters becomes very important to use durable asphalt mixture in the surface layer, e.g., SMA (vale et al. 2006).

These SMA mixtures are gap graded mixes with high structural and functional performance. They are discontinuous gradations with higher coarse aggregates of about 70-80% by weight to give stone to stone contact (Skelton like structure) of high stability with high binder content of about 6-7% and air voids approximately 4%, it requires the addition of fibers of about 0.3% of weight of the mix to prevent the drain down through the air voids.

This research aims to determine the feasibility of using reclaimed asphalt pavement in SMA mix, due to more percentage of RAP materials available in the southern part of India. RAP material constitute the potential alternative material to natural aggregates. To evaluate the performance characteristics of SMA mix with elastomer modifier, following tests have been carried such as marshall stability, indirect tensile strength, rutting, fatigue, drain down and moisture susceptibility. The obtained test results are compared with conventional SMA mix, SMA mix with RSP and SMA mix with RAP and elastomer modifier.

BACKGROUND

Stone mastic asphalt (SMA) was first used by Germans in 1960's as a mix would resist the wear and tear of studded tyre. Main advantages of SMA were found to be improved low-temperature performance, highly rut resist, reduced noise of tires and increased service life (cooley and Hurley 2004). Due to its successful performance in Germany the mix started to use in different part of the world like USA, Spain, France and North America. SMA application has been carried mainly on the high-volume roads as a high resistance and high durable layer which helps in noise reduction and light reflection during rainy season.

A typical SMA mix consists 70% of coarse aggregates on or above 2.36mm sieve, filler in SMA is about 10% of weight of mix passing on 0.075mm sieve (Dong and Tan 2011).cellulose fibre of about 0.3% of weight is added to prevent drain down, if fibers are not added to the mix it will leads formation of fat patch on the pavement surface (Michael et al. 2003) stone on stone contact formed in the mix makes the more rut resist and highly durable mixture.

Stabilizer additive, Fibers are added due to high binder content, drain down of bitumen from SMA mix during production, transport and laying of the mix occurs. Hence, in order to prevent the drain down fibers are added which may be organic or inorganic or mineral materials. Fibers such as coconut coir, glass, minerals and cellulose fibers (only pelletized) can be utilized.(Neves Filho et al.2004). Addition of fibers should not exceed more than 0.3% by weight of the mix. Mix is designed as

per ASTM D 6390. Length of fiber is about 8mm, ash content is max 20% nonvolatile, oil absorption is 4 times more than the fiber weight and moisture content should be less than 5% then by weight of the fiber.

Many researchers have show that SMA mix design been developed by marshall mix design method (Muniandy and Huat 2006) by adopting marshall method of mix design performance results such as dynamic stability, fatigue life and water sensitivity of SMA mix is evaluated.

In the present research studies RAP is replaced with natural aggregated by adopting marshall mix design to evaluate the performance characteristics of SMA mix.

MATERIALS

Aggregates and Filler

In the present study for the design and production SMA mix, granite and dust of rock was used, with bag house dust as a filler. The natural aggregates were procured from KMS crusher-Bagalur, Karnataka India. Filler material used in the research is bag house. Which, is the final product obtained by washing stone dust during production of M-sand. Bag house for the research is provided by Sri venkateshwara crushes, Bangalore, Karnataka, India.

Fibers

Most commonly used fiber in SMA mix is cellulose which are physiologically and toxicologically safe, and they are purely natural cellulose resources (Bose et al. 2006). Small amount of about 0.3% is added to the mix to prevent drain down of asphalt and to increase stability and durability of the mix. Cellulose fiber used in the present research is provided by SMART Technology, Peenya, Bangalore, Karnataka, India. and Table 3 shows the properties of fiber which is used for the SMA mix.

Reclaimed Asphalt Pavement

Reclaimed asphalt pavement are the materials derived from reprocessed pavement which contains asphalt and aggregates. The derived materials are subjected to crushing and screening to obtain RAP of well graded aggregates coated with bitumen. Recycling of existing asphalt pavements allows the industry to produce new pavements with effective utilization of material and energy making it cost effective [7]. For this study, RAP is procured from Chittoor district where some part of Bengaluru-Tirupati Highway (NH-206) was scarified for construction of cement concrete pavement.

Bitumen (CRMB 55)

Binder content required for the SMA mix is higher when compared to other bituminous mixes. In this studies Crumb Rubber Modified Bitumen of grade 55 (CRMB-55) is used. CRMB was procured from the M D Constructions, Rajajinagar, Bengaluru. Procured bitumen is tested for penetration, softening point, flash point, elastic recovery and results obtained should meet the required specifications as per IRC SP: 53:2010.

Table 1: Physical properties of natural aggregates and RAP aggregates

Properties	Natural aggregates	RAP aggregates
Combined flakiness & Elongation indices, %	11.19	16.83
Aggregate Impact value, %	16.08	16.27
Los Angeles abrasion value, %	17.52	19.84
Water absorption, %	0.2	0.55
Specific gravity		
20 mm down	2.68	2.62
12 mm down	2.69	2.66
6 mm down	2.71	2.69
Stone dust	2.72	-

Table 2: Properties of CRMB-55

Properties	Result	Test methods
Penetration value at 25 °C (mm)	57	IS 1203-1978
Softening point (°C)	56	IS 1205-1978
Specific gravity	1.03	IS 1202-1978
Flash Point (°C)	279	IS 1209-1978
Elastic Recovery in ductilometer at 15°C, (%)	68	-
Thin Film Oven tests and test on residue:		
a) Percentage loss in mass	0.115	IS – 9382
b) Increase in softening point value, (°C)	4	IS – 1205
c) Reduction in penetration of residue, at 25 °C, (%)	33	IS – 1203
d) Elastic recovery in ductilometer at 25 °C, (%)	56	-
Separation difference in softening point, max, (°C)	2.85	-

Filler Material

In the present research work Baghouse dust is used as mineral filler in SMA mix. Baghouse mineral filler used for the study was obtained from S L V Crushers, Periyapatna, and Karnataka. The gradation is carried out as per MoRTH Table 500-36.

Table 3: Properties of filler

Sl.no	Test	Result
1	Specific Gravity	2.59
2	Fines by 75 μ sieve	90%

Waste Engine Oil (WEO)

Reclaimed asphalt pavement (RAP) consists of aggregates coated with the bitumen content, the coated binder will be of aged one and its properties are not as good as original binder. The aged binder present in RAP will be lack in oily compounds and doesn't behave as fresh bitumen. In the present study Waste Engine Oil is used as a rejuvenators and which was procured from the motor cycle Garage Bengaluru.

MARSHALL MIX DESIGN

Once the gradation is fixed, Marshall Method is used to determine the OBC (optimum binder content) in SMA mix. The SMA mix will be designed using AASTHO MP8 standard specification for designing SMA and AASHTO PP41. For preparing Marshall Samples 50 blows are given on both side of the sample. The optimum binder content shall be determined by corresponding to the design air void content V_a of 4.0% and minimum VMA of 17%. The minimum bitumen content for SMA mix is 6% by the weight of the mix. The results were tabulated in Table 4.

Table 4: Marshall test Results

Types of mixes	OBC (%)	Density (kg/m ³)	Stability (kN)	Flow (mm)	Airvoids (%)	Voids in minral aggregates (%)
Conventional SMA mix	6.3	2450	11.108	3.187	4.1	17.64
SMA mix with 60%RAP replacement without WEO	6.2	2430	12.65	3.8	3.99	17.38
SMA mix with 70%RAP and 25% WEO	6.2	2430	13.01	3.14	3.99	18.27

MOISTURE SUSCEPTIBILITY AND INDIRECT TENSILE STRENGTH (ITS)

Moisture susceptibility is done in order to know the ability of SMA mix to resist the moisture damage. Around 6 samples were prepared, 3 samples were tested for conditioned and 3 were tested for controlled. The compaction of the specimen should be so, that it should have air voids $7 \pm 0.5\%$. Here controlled samples were subjected to freeze-thaw cycle. Then specimens are tested for ITS. The TSR (Tensile Strength Ratio) for SMA mix should be minimum 85% ([IRC:SP:79-2008](#)).

The Indirect Tensile Strength (ITS) test was conducted as per ASTM D4123. Here, the specimens were loaded diametrically and the load at failure was noted down. The failure load obtained from the test was used to fix the stress level in the repeated fatigue load test.

Tensile Strength of each specimen were calculated as.

$$St = \frac{2000P}{\pi td}$$

Where, St = tensile strength, P = maximum loads (N), t = specimen thickness (mm), d = specimen diameter (mm)

Calculating the Tensile Strength Ratio (TSR) as follows,

$$TSR = S2/S1$$

Where, S1 = average tensile strength of controlled subset (kPa), S2 = average tensile strength of conditioned subset (kPa).

Table 5 Indirect tensile strength ratio

Types of mixes	Moisture susceptibility (kPa)		Tensile strength ratio (TSR %)
	Unconditioned	Conditioned	
Conventional SMA mix	340.66	312.86	91.84
SMA mix with 60%RAP replacement without WEO	776.48	724.49	93.30
SMA mix with 70%RAP and 25% WEO	388.42	377.65	97.23

REPEATED LOAD FATIGUE TEST

Pavement distress of flexible pavements due to repeated application of heavy wheel loads is one of the major pavement failures. It is necessary to find the fatigue behaviour of the bituminous mix under repeated dynamic load as in-situ. The diametrical dynamic loading fatigue test is an indirect tensile test on a cylindrical specimen in which a repeating compressive load is applied along the vertical diametrical plane in a half sine wave pattern. The test is conducted for conventional SMA mix, SMA mix with optimum RAP material content and SMA with RAP material and bitumen modifier at 10%, 20%, 30% and 40% of failure load which is obtained from ITS test. The apparatus is shown in below figure consist of LVDT (Linear variable differential transducer), loading piston, loading frame, position controller and data acquisition system with a loading rate capacity up to 5 tonnes. Load is applied over the specimen through two stainless steel plates using loading frame. When the specimen starts deforming, then horizontal and vertical deformation of the sample were measured using two sets of (i.e 4no) of LVDT's. The recording job of the applied load, deflection of both horizontal and vertical LVDT's for every cycle and the total number of repetitions before failure for each individual test was done by a data acquisition system. In the present study, the following test conditions were adopted such as loading frequency of 1 Hz (1 cycle per second), failure deformation of 5mm, pressure around 30-35 Pascals. The following expressions from Kennedy (1978), Gupta and Veeraragavan (2009) etc. were used for the computation of resilient modulus and initial tensile strain values from the experimental data.

Resilient poisson ratio is calculated using,

$$\mu_r = 3.59 \cdot (H_r/V_r) - 0.27$$

The resilient modulus is calculated using,

$$M_r = P \cdot (0.27 + \mu_r) / (H_r \cdot h)$$

Initial tensile strain,

$$\epsilon = \sigma_i (1 + 3 \mu_r) / M_r$$

The maximum horizontal tensile stress is calculated using,

$$\sigma_{max} = 2P / \pi d t$$

Where, μ_r = Resilient Poisson ratio, H_r = average horizontal deformation in mm,

V_r = average vertical deformation in mm, M_r = Resilient Modulus of Elasticity (MPa),

P = applied load in N, h = height of specimen in mm, d = diameter of specimen in mm,

t = thickness of specimen in mm

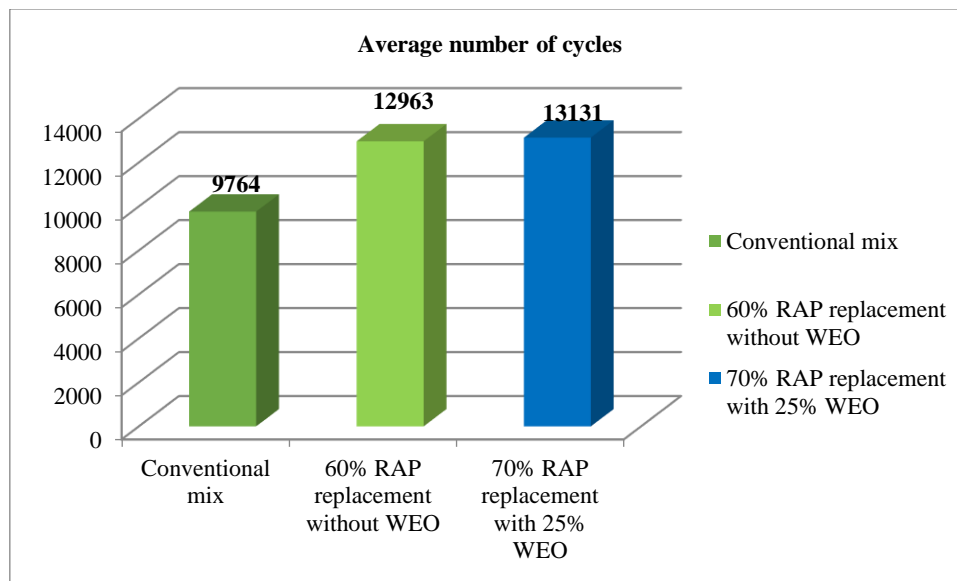


Fig 1 Fatigue test Results

Resistance to rutting

Immersion wheel tracking test is carried out to examine the permanent deformation characteristics of SMA mix. One of the main reasons for premature failure of pavement is rut formation due to movement of heavy traffic loads. Immersion wheel tracking machine consists of loaded wheel of 5.6 kg/cm² pressure and confined mould of dimension 60*10*20 cm in which specimen was placed and it was confined from all sides. In immersion wheel tracking machine, wheel is made to move back and forth on the specimen at the rate of 25 passes per minute which exactly simulates the practical condition of vehicles movement on pavement. Specimens were subjected 10000 passes and rut depths at regular intervals were noted down. Graphs are plotted number of passes versus rut depth for 45 and 75mm thickness specimens and were shown below.

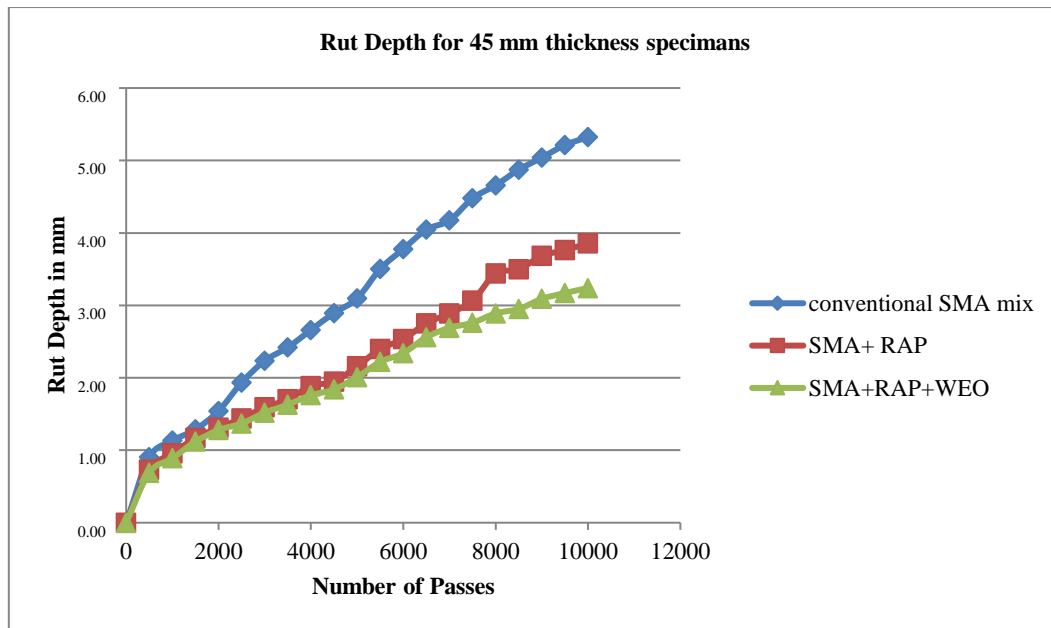


Fig 2 Rutting Characteristics for 45 mm thickness specimens

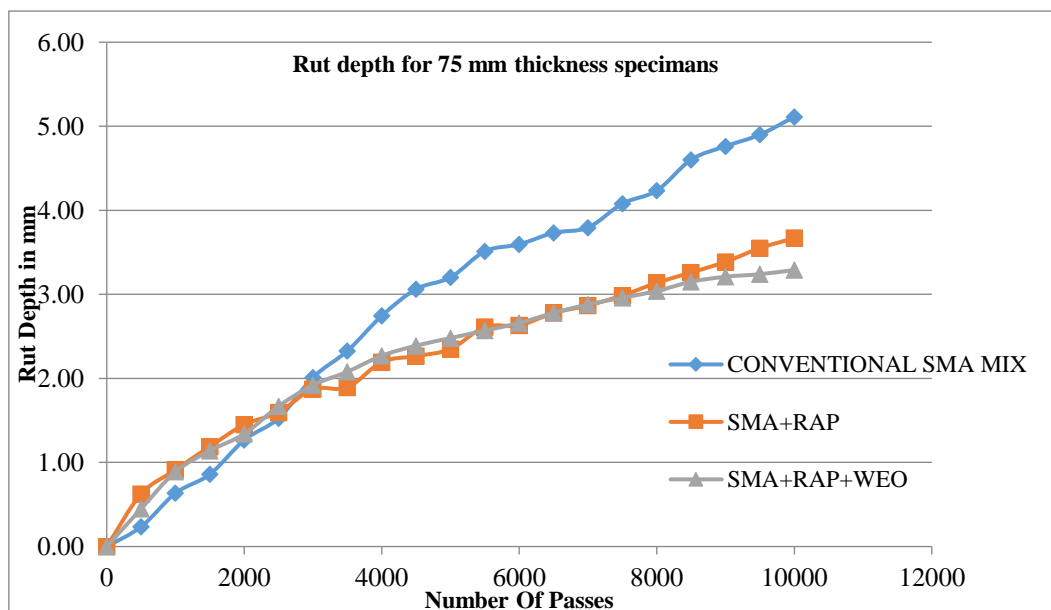


Fig 3 Rutting Characteristics for 75 mm thickness specimens

CONCLUSIONS

- Laboratory tests have been carried out in order to determine the physical properties of natural aggregate, Crumb Rubber Modified Bitumen (CRMB), baghouse dust and the results obtained were found to be well within the specified limits as per MoRTH.
- From the marshal stability test optimum binder content for conventional SMA mix is found to be 6.3%.
- From marshal test results it is observed that optimum RAP replacement percentage for SMA mix with RAP and WEO (i.e. 70%) is high compared to SMA mix using RAP without waste engine oil (i.e. 60%) at OBC 6.2%.
- Draindown test results obtained for conventional SMA mix, SMA mix using RAP without WEO, and SMA mix using RAP and WEO are found to be within the limit as per the MoRTH.
- From the Moisture susceptibility test results it is found that SMA mix with 70% RAP and 25% WEO shows higher TSR values compared to conventional SMA mix and SMA mix with 60% RAP without adding waste engine oil.
- From rutting test results it is observed that SMA mix using 70% RAP and 25% waste engine oil shows higher rut resisting property when compared to conventional SMA mix and SMA mix with 60% RAP without adding waste engine oil.

- From the fatigue test results, it is observed that SMA mix with 70% of RAP and 25% of waste engine oil shows more fatigue life value when compared to conventional SMA mix and SMA mix with 60% RAP without waste engine oil.
- From the cost analysis, it is observed that cost per cubic meter for SMA mix using RAP and WEO is less because of high percentage of RAP replacement when compared with conventional SMA mix and SMA mix with 60% RAP without waste engine oil.
- Hence by using RAP as an alternative material along with rejuvenator makes the pavement sustainable, durable and economical.

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